STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

ROY COOPER
GOVERNOR

JAMES H. TROGDON, III
SECRETARY

February 18, 2020

Addendum No. 1

RE: **DD00326**
Northampton and Halifax Counties

To Whom It May Concern:

Reference is made to the above-mentioned project. The following changes/additions/deletions have been made:

1. The Notes to Contractor were revised to address items discussed in the Pre-Bid conference. Changes are highlighted in gray. Please replace Page 5 with the attached Pages 5 and 5-A.
2. The Structure Special Provision was revised to clarify items discussed in the Pre-Bid conference. Per the Special Provision, the entire strip seal assembly (frame, gland, anchor bolts, adhesive) shall be furnished by NCDOT. Please replace Pages 49 through 65 with the attached Pages that will become Pages 49 through 65-A & 65-B.
3. The line item "Grooving Bridge Floors" has been deleted. Please replace Page 99 with the attached Page.

This sheet and attachments shall be made a part of the plans and bid documents and shall be submitted with the bid. Bids submitted without the addenda and attachments will be considered non-responsive.

If there are any questions, please contact me at (252) 640-6433.

Sincerely,

DocuSigned by:
Vickie P. Gardner
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2/18/2020

Vickie P. Gardner
Division Four Proposals Engineer

VPG/
Attachments

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509 WARD BOULEVARD
WILSON, NC 27895

PROJECT SPECIAL PROVISIONS

GENERAL

NOTES TO CONTRACTOR:

The scope of work for this project includes:

1. Remove existing Molded Rubber Segmental Expansion Joints and install new Department Furnished Strip Seals on Halifax Bridges 139 and 141 and Northampton Bridges 009 and 011 as detailed in the plans and proposal.

2. Install temporary and permanent pavement markings on Halifax Bridges 124 and 129 (I-95 over US 158), 131 and 132 (I-95 over CSX Railroad and Becker Drive), 139 and 141 (I-95 over Roanoke River) and Northampton Bridges 009 and 011 (I-95 over Roanoke River).

3. Provide Traffic Control and incidentals necessary to complete the work.

The Department shall furnish the entire Strip Seal assemblies as well as the Epoxy Coated Reinforcing Steel that is shown in the plans as being necessary to complete the work. Mechanical couplers are provided for use in the installation of reinforcing bars. A Strip Seal assembly shall be defined as the steel retaining rails, neoprene seal gland, adhesive for gland installation, barrier rail cover plates, and anchoring bolts. Any missing components of the Strip Seal assemblies, as defined above, shall be the responsibility of the Contractor to furnish and shall be handled in accordance to Article 104-7 of the Standard Specifications. All other materials required to complete the installation shall be considered incidental to the work.

The concrete surfaces shall be hand-tined to an acceptable finish in accordance with the Special Provisions, as determined by the Engineer. This work shall be considered incidental to the placement and finishing of the concrete.

It is the intent of this contract to specify the use of prepackaged concrete materials to be mixed onsite with an approved volumetric mixer, and to comply with the requirements of the Special Provisions.

The Division shall allow the use of pneumatic hammers up to 90lbs for the demolition of concrete in areas that require the full-depth removal of the concrete deck, with the following exceptions. Hammer size will be limited to a maximum of 17lbs in areas that are within 6” of the limits of concrete deck removal or above concrete diaphragms.

In order to maintain traffic, installation of the Department Furnished Strip Seals must be phased per lane in a given direction of travel.

The Contractor shall install Paint Pavement Marking Lines 6” early in the life of the project on the bridge decks listed above and as deemed necessary by the Engineer to ensure adequate markings are in place for the traveling public.

Upon completion of the Bridge Joint work, the Contractor shall install Polyurea Pavement Marking Lines 6", 20 mils on all bridge decks listed above. At the Contractors option Polyurea Pavement Marking lines 6", 20 mils may be placed on Halifax Bridges 124 and 129 (I-95 over US 158) and Halifax Bridges 131 and 132 (I-95 over CSX Railroad and Becker Drive) early in the life of the project in lieu of the initial Paint Pavement Marking Lines installation.

Department furnished Strip Seal and Epoxy Coated Reinforcing Steel materials are located and stored at the Halifax County Bridge Maintenance Yard, 14065 Hwy. 903 Halifax, NC 27839.

Due to the ongoing deterioration of the existing Molded Rubber Segmental Expansion Joints it is the Departments desire to complete installation of as many Strip Seals as possible prior to May 15, 2020. Please coordinate with the Engineer regarding any preferred order of joint replacement.

Project Special Provisions
Structures
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Aster Abraha
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2/17/2020

STRUCTURE PROJECT SPECIAL PROVISIONS

SCOPE OF WORK

This work shall consist of furnishing all labor, equipment, and materials to repair joints. Work includes: portable lighting; installation of strip joint seals; Deck repair using concrete; preparation and disposal of demolition and waste material; seeding and mulching all grassed areas disturbed; and all incidental items necessary to complete the project as specified and shown on the plans. No separate payment will be made for portable lighting as the cost of such is incidental to the work being performed.

Work will be performed on the existing bridge at the following location:

1.) Halifax County Bridge No. 139 – I-95 NBL over Roanoke River
2.) Halifax County Bridge No. 141 – I-95 SBL over Roanoke River
3.) Northampton County Bridge No. 009 – I-95 NBL over Roanoke River
4.) Northampton County Bridge No. 011 – I-95 SBL over Roanoke River

The contractor shall provide all necessary access; underdeck platforms, scaffolding, ladders, etc.; provide all traffic control; provide all staging areas, material storage, waste disposal; provide environmental controls to limit loss of materials from sawing equipment, and chipping equipment; and all else necessary to complete the work.

The contractor shall be responsible for fulfilling all requirements of the NCDOT Standard Specifications for Roads and Structures dated January 2018, except as otherwise specified herein.

FALSEWORK AND FORMWORK (4-5-12)

1.0 DESCRIPTION

Use this Special Provision as a guide to develop temporary works submittals required by the Standard Specifications or other provisions; no additional submittals are required herein. Such temporary works include, but are not limited to, falsework and formwork.

Falsework is any temporary construction used to support the permanent structure until it becomes self-supporting. Formwork is the temporary structure or mold used to retain plastic or fluid concrete in its designated shape until it hardens. Access scaffolding is a temporary structure that functions as a work platform that supports construction personnel, materials, and tools, but is not intended to support the structure. Scaffolding systems that are used to temporarily support permanent structures (as opposed to functioning as work platforms) are considered to be falsework under the definitions given. Shoring is a component of falsework

such as horizontal, vertical, or inclined support members. Where the term "temporary works" is used, it includes all of the temporary facilities used in bridge construction that do not become part of the permanent structure.

Design and construct safe and adequate temporary works that will support all loads imposed and provide the necessary rigidity to achieve the lines and grades shown on the plans in the final structure.

2.0 MATERIALS

Select materials suitable for temporary works; however, select materials that also ensure the safety and quality required by the design assumptions. The Engineer has authority to reject material on the basis of its condition, inappropriate use, safety, or nonconformance with the plans. Clearly identify allowable loads or stresses for all materials or manufactured devices on the plans. Revise the plan and notify the Engineer if any change to materials or material strengths is required.

3.0 DESIGN REQUIREMENTS

A. Working Drawings

Provide working drawings for items as specified in the contract, or as required by the Engineer, with design calculations and supporting data in sufficient detail to permit a structural and safety review of the proposed design of the temporary work.

On the drawings, show all information necessary to allow the design of any component to be checked independently as determined by the Engineer.

When concrete placement is involved, include data such as the drawings of proposed sequence, rate of placement, direction of placement, and location of all construction joints. Submit the number of copies as called for by the contract.

When required, have the drawings and calculations prepared under the guidance of, and sealed by, a North Carolina Registered Professional Engineer who is knowledgeable in temporary works design.

If requested by the Engineer, submit with the working drawings manufacturer's catalog data listing the weight of all construction equipment that will be supported on the temporary work. Show anticipated total settlements and/or deflections of falsework and forms on the working drawings. Include falsework footing settlements, joint take-up, and deflection of beams or girders.

As an option for the Contractor, overhang falsework hangers may be uniformly spaced, at a maximum of 36 inches, provided the following conditions are met:

Member Type (PCG)	Member Depth, (inches)	Max. Overhang Width, (inches)	Max. Slab Edge Thickness, (inches)	Max. Screed Wheel Weight, (lbs.)	Bracket Min. Vertical Leg Extension, (inches)
II	36	39	14	2000	26

III	45	42	14	2000	35
IV	54	45	14	2000	44
MBT	63	51	12	2000	50
MBT	72	55	12	1700	48

Overhang width is measured from the centerline of the girder to the edge of the deck slab.

For Type II, III & IV prestressed concrete girders (PCG), 45-degree cast-in-place half hangers and rods must have a minimum safe working load of 6,000 lbs.

For MBT prestressed concrete girders, 45-degree angle holes for falsework hanger rods shall be cast through the girder top flange and located, measuring along the top of the member, 1'-2 ½" from the edge of the top flange. Hanger hardware and rods must have a minimum safe working load of 6,000 lbs.

The overhang bracket provided for the diagonal leg shall have a minimum safe working load of 3,750 lbs. The vertical leg of the bracket shall extend to the point that the heel bears on the girder bottom flange, no closer than 4 inches from the bottom of the member. However, for 72-inch members, the heel of the bracket shall bear on the web, near the bottom flange transition.

Provide adequate overhang falsework and determine the appropriate adjustments for deck geometry, equipment, casting procedures and casting conditions.

If the optional overhang falsework spacing is used, indicate this on the falsework submittal and advise the girder producer of the proposed details. Failure to notify the Engineer of hanger type and hanger spacing on prestressed concrete girder casting drawings may delay the approval of those drawings.

Falsework hangers that support concentrated loads and are installed at the edge of thin top flange concrete girders (such as bulb tee girders) shall be spaced so as not to exceed 75% of the manufacturer's stated safe working load. Use of dual leg hangers (such as Meadow Burke HF-42 and HF-43) are not allowed on concrete girders with thin top flanges. Design the falsework and forms supporting deck slabs and overhangs on girder bridges so that there will be no differential settlement between the girders and the deck forms during placement of deck concrete.

When staged construction of the bridge deck is required, detail falsework and forms for screed and fluid concrete loads to be independent of any previous deck pour components when the mid-span girder deflection due to deck weight is greater than ¾".

Note on the working drawings any anchorages, connectors, inserts, steel sleeves or other such devices used as part of the falsework or formwork that remains in the permanent structure. If the plan notes indicate that the structure contains the necessary corrosion protection required for a Corrosive Site, epoxy coat, galvanize or metalize these devices. Electroplating will not be allowed. Any coating required by the Engineer will be considered incidental to the various pay items requiring temporary works.

Design falsework and formwork requiring submittals in accordance with the 1995 AASHTO *Guide Design Specifications for Bridge Temporary Works* except as noted herein.

1. Wind Loads

Table 2.2 of Article 2.2.5.1 is modified to include wind velocities up to 110 mph. In addition, Table 2.2A is included to provide the maximum wind speeds by county in North Carolina.

Table 2.2 - Wind Pressure Values

Height Zone feet above ground	Pressure, lb/ft^2 for Indicated Wind Velocity, mph				
	70	80	90	100	110
0 to 30	15	20	25	30	35
30 to 50	20	25	30	35	40
50 to 100	25	30	35	40	45
over 100	30	35	40	45	50

2. Time of Removal

The following requirements replace those of Article 3.4.8.2.

Do not remove forms until the concrete has attained strengths required in Article 420-16 of the Standard Specifications and these Special Provisions.

Do not remove forms until the concrete has sufficient strength to prevent damage to the surface.

Table 2.2A - Steady State Maximum Wind Speeds by Counties in North Carolina

COUNTY	25 YR (mph)	COUNTY	25 YR (mph)	COUNTY	25 YR (mph)
Alamance	70	Franklin	70	Pamlico	100
Alexander	70	Gaston	70	Pasquotank	100
Alleghany	70	Gates	90	Pender	100
Anson	70	Graham	80	Perquimans	100
Ashe	70	Granville	70	Person	70
Avery	70	Greene	80	Pitt	90
Beaufort	100	Guilford	70	Polk	80
Bertie	90	Halifax	80	Randolph	70
Bladen	90	Harnett	70	Richmond	70
Brunswick	100	Haywood	80	Robeson	80
Buncombe	80	Henderson	80	Rockingham	70
Burke	70	Hertford	90	Rowan	70

Cabarrus	70	Hoke	70	Rutherford	70
Caldwell	70	Hyde	110	Sampson	90
Camden	100	Iredell	70	Scotland	70
Carteret	110	Jackson	80	Stanley	70
Caswell	70	Johnston	80	Stokes	70
Catawba	70	Jones	100	Surry	70
Cherokee	80	Lee	70	Swain	80
Chatham	70	Lenoir	90	Transylvania	80
Chowan	90	Lincoln	70	Tyrell	100
Clay	80	Macon	80	Union	70
Cleveland	70	Madison	80	Vance	70
Columbus	90	Martin	90	Wake	70
Craven	100	McDowell	70	Warren	70
Cumberland	80	Mecklenburg	70	Washington	100
Currituck	100	Mitchell	70	Watauga	70
Dare	110	Montgomery	70	Wayne	80
Davidson	70	Moore	70	Wilkes	70
Davie	70	Nash	80	Wilson	80
Duplin	90	New Hanover	100	Yadkin	70
Durham	70	Northampton	80	Yancey	70
Edgecombe	80	Onslow	100		
Forsyth	70	Orange	70		

B. Review and Approval

The Engineer is responsible for the review and approval of temporary works' drawings.

Submit the working drawings sufficiently in advance of proposed use to allow for their review, revision (if needed), and approval without delay to the work.

The time period for review of the working drawings does not begin until complete drawings and design calculations, when required, are received by the Engineer.

Do not start construction of any temporary work for which working drawings are required until the drawings have been approved. Such approval does not relieve the Contractor of the responsibility for the accuracy and adequacy of the working drawings.

4.0 CONSTRUCTION REQUIREMENTS

All requirements of Section 420 of the Standard Specifications apply.

Construct temporary works in conformance with the approved working drawings. Ensure that the quality of materials and workmanship employed is consistent with that assumed in the design of the temporary works. Do not weld falsework members to any portion of the

permanent structure unless approved. Show any welding to the permanent structure on the approved construction drawings.

Provide tell-tales attached to the forms and extending to the ground, or other means, for accurate measurement of falsework settlement. Make sure that the anticipated compressive settlement and/or deflection of falsework does not exceed 1 inch. For cast-in-place concrete structures, make sure that the calculated deflection of falsework flexural members does not exceed 1/240 of their span regardless of whether or not the deflection is compensated by camber strips.

A. Maintenance and Inspection

Inspect and maintain the temporary work in an acceptable condition throughout the period of its use. Certify that the manufactured devices have been maintained in a condition to allow them to safely carry their rated loads. Clearly mark each piece so that its capacity can be readily determined at the job site.

Perform an in-depth inspection of an applicable portion(s) of the temporary works, in the presence of the Engineer, not more than 24 hours prior to the beginning of each concrete placement. Inspect other temporary works at least once a month to ensure that they are functioning properly. Have a North Carolina Registered Professional Engineer inspect the cofferdams, shoring, sheathing, support of excavation structures, and support systems for load tests prior to loading.

B. Foundations

Determine the safe bearing capacity of the foundation material on which the supports for temporary works rest. If required by the Engineer, conduct load tests to verify proposed bearing capacity values that are marginal or in other high-risk situations.

The use of the foundation support values shown on the contract plans of the permanent structure is permitted if the foundations are on the same level and on the same soil as those of the permanent structure.

Allow for adequate site drainage or soil protection to prevent soil saturation and washout of the soil supporting the temporary works supports.

If piles are used, the estimation of capacities and later confirmation during construction using standard procedures based on the driving characteristics of the pile is permitted. If preferred, use load tests to confirm the estimated capacities; or, if required by the Engineer conduct load tests to verify bearing capacity values that are marginal or in other high-risk situations.

The Engineer reviews and approves the proposed pile and soil bearing capacities.

5.0 REMOVAL

Unless otherwise permitted, remove and keep all temporary works upon completion of the work. Do not disturb or otherwise damage the finished work.

Remove temporary works in conformance with the contract documents. Remove them in such a manner as to permit the structure to uniformly and gradually take the stresses due to its own weight.

6.0 METHOD OF MEASUREMENT

Unless otherwise specified, temporary works will not be directly measured.

7.0 BASIS OF PAYMENT

Payment at the contract unit prices for the various pay items requiring temporary works will be full compensation for the above falsework and formwork.

CRANE SAFETY (6-20-19)

Comply with the manufacturer specifications and limitations applicable to the operation of any and all cranes and derricks. Prime contractors, sub-contractors, and fully operated rental companies shall comply with the current Occupational Safety and Health Administration (OSHA) regulations.

Submit all items listed below to the Engineer prior to beginning crane operations. Changes in personnel or equipment must be reported to the Engineer and all applicable items listed below must be updated and submitted prior to continuing with crane operations.

CRANE SAFETY SUBMITTAL LIST

A. **Competent Person:** Provide the name and qualifications of the "Competent Person" responsible for crane safety and lifting operations. The named competent person will have the responsibility and authority to stop any work activity due to safety concerns.

B. **Riggers:** Provide the qualifications and experience of the persons responsible for rigging operations. Qualifications and experience should include, but not be limited to, weight calculations, center of gravity determinations, selection and inspection of sling and rigging equipment, and safe rigging practices.

C. **Crane Inspections:** Inspection records for all cranes shall be current and readily accessible for review upon request.

D. **Certifications:** Crane operators shall be certified by the National Commission for the Certification of Crane Operators (NCCCO) or the National Center for Construction Education and Research (NCCER). Other approved nationally accredited programs will be considered upon request. In addition, crane operators shall have a current CDL medical card. Submit a list of crane operator(s) and include current certification for each

type of crane operated (small hydraulic, large hydraulic, small lattice, large lattice) and medical evaluations for each operator.

GROUT FOR STRUCTURES (9-30-11)

1.0 DESCRIPTION

This special provision addresses grout for use in pile blockouts, grout pockets, shear keys, dowel holes and recesses for structures. This provision does not apply to grout placed in post-tensioning ducts for bridge beams, girders, or decks. Mix and place grout in accordance with the manufacturer's recommendations, the applicable sections of the Standard Specifications and this provision.

2.0 MATERIAL REQUIREMENTS

Use a Department approved pre-packaged, non-shrink, non-metallic grout. Contact the Materials and Tests Unit for a list of approved pre-packaged grouts and consult the manufacturer to determine if the pre-packaged grout selected is suitable for the required application.

When using an approved pre-packaged grout, a grout mix design submittal is not required.

The grout shall be free of soluble chlorides and contain less than one percent soluble sulfate. Supply water in compliance with Article 1024-4 of the Standard Specifications.

Aggregate may be added to the mix only where recommended or permitted by the manufacturer and Engineer. The quantity and gradation of the aggregate shall be in accordance with the manufacturer's recommendations.

Admixtures, if approved by the Department, shall be used in accordance with the manufacturer's recommendations. The manufacture date shall be clearly stamped on each container. Admixtures with an expired shelf life shall not be used.

The Engineer reserves the right to reject material based on unsatisfactory performance.

Initial setting time shall not be less than 10 minutes when tested in accordance with ASTM C266.

Test the expansion and shrinkage of the grout in accordance with ASTM C1090. The grout shall expand no more than 0.2% and shall exhibit no shrinkage. Furnish a Type 4 material certification showing results of tests conducted to determine the properties listed in the Standard Specifications and to assure the material is non-shrink.

Unless required elsewhere in the contract the compressive strength at 3 days shall be at least 5000 psi. Compressive strength in the laboratory shall be determined in accordance with ASTM C109 except the test mix shall contain only water and the dry manufactured material. Compressive strength in the field will be determined by molding and testing 4" x 8" cylinders in accordance with AASHTO T22. Construction loading and traffic loading shall not be allowed until the 3-day compressive strength is achieved.

When tested in accordance with ASTM C666, Procedure A, the durability factor of the grout shall not be less than 80.

3.0 SAMPLING AND PLACEMENT

Place and maintain components in final position until grout placement is complete and accepted. Concrete surfaces to receive grout shall be free of defective concrete, laitance, oil, grease and other foreign matter. Saturate concrete surfaces with clean water and remove excess water prior to placing grout.

Do not place grout if the grout temperature is less than 50°F or more than 90°F or if the air temperature measured at the location of the grouting operation in the shade away from artificial heat is below 45°F.

Provide grout at a rate that permits proper handling, placing and finishing in accordance with the manufacturer's recommendations unless directed otherwise by the Engineer. Use grout free of any lumps and undispersed cement. Agitate grout continuously before placement.

Control grout delivery so the interval between placing batches in the same component does not exceed 20 minutes.

The Engineer will determine the locations to sample grout and the number and type of samples collected for field and laboratory testing. The compressive strength of the grout will be considered the average compressive strength test results of 3 cube or 2 cylinder specimens at 28 days.

4.0 BASIS OF PAYMENT

No separate payment will be made for "Grout for Structures". The cost of the material, equipment, labor, placement, and any incidentals necessary to complete the work shall be considered incidental to the structure item requiring the grout.

STRIP SEALS (SPECIAL)

1.0 GENERAL

This Special Provision covers installing strip seal expansion joints as shown on the contract drawings and in accordance with this Special Provision, the Standard Specifications and the manufacturer's recommendation. Strip Seals will be furnished by NCDOT, all other materials, labor, equipment, and incidentals necessary for proper installation of the strip seal expansion joints are included in this provision.

2.0 MATERIALS

NCDOT will furnish strip seal expansion joints capable of accommodating a total movement measured parallel to the centerline of the roadway as shown on plans. The components of the expansion joint include steel retainer rails and a neoprene seal gland.

The steel retainer rails consist of a "P" shape profile configuration with anchor studs welded to the concrete face. The rails shall have a minimum height of 8 inches, a minimum thickness of ½ inch and a maximum top surface (at the riding surface) width of 2 inches. Use AASHTO M270 Grade 36 or Grade 50 steel for the steel retainer rails.

The neoprene gland is extruded synthetic rubber with virgin polychloroprene as the only polymer. The gland manufacturer has provided a Type 4 certification, in accordance with the Standard Specifications, attesting the gland has been tested and meets the following minimum properties:

PHYSICAL PROPERTY	TEST METHOD	REQUIREMENTS
Tensile Strength, psi (min.)	ASTM D412	2000
Elongation at break, % (min.)	ASTM D412	250
Hardness, Type A durometer, points	ASTM D2240 Modified	60 ± 10
Oven aging, 70h @ 212°F Tensile strength, % change (max.) Elongation, % change (max.) Hardness, points change (max.)	ASTM D573	 -20 –20 0 to +10
Oil Swell, ASTM Oil No. 3, 70h @ 212°F Weight change, % (max.)	ASTM D471	 45
Ozone resistance 20% strain, 300 pphm in air 70h @ 104°F	ASTM D1149 Modified	No cracks
Low temperature stiffening, 7 days @ 14°F Hardness, Type A durometer, points change	ASTM D2240	0 to +15
Compression Set, 70h @ 212°F (max.)	ASTM D395 Method B (modified)	40%

3.0 SHOP DRAWINGS

Shop Drawings shall be furnished by NCDOT, upon request.

4.0 SHIPMENT

Strip seals shall be furnished by NCDOT. Any portion of the strip seal assembly not provided by the Department shall be provided by the Contractor. Payment for additional necessary strip seal materials, not provided by the Department, will be paid for in accordance with Article 104-7 of the Standard Specifications. This does not apply to incidentals needed to install the joint assembly, but only the actual hardware items of the strip seal device.

5.0 INSTALLATION

Install the strip seal expansion joint in accordance with the plan details, this Special Provision, the Standard Specifications, and the manufacturer's recommended installation procedures. Have a manufacturer's representative present onsite for the setting of the steel retainer rails and placement of the concrete around rails of the first joint, and as required by the Engineer. Additionally, have a manufacturer's representative present onsite during the first gland installation, and as required by the Engineer.

Install the steel retainer rail assemblies at proper grade and alignment. See contract drawings for width of joint opening.

Bolt, weld or clamp steel retainer rail assemblies in position using temporary or sacrificial brackets as required. Do not use temporary or sacrificial support brackets, bolts, clamps, etc. between the faces of the steel retainer rails. Do not weld within 2 inches of steel retainer rail surfaces exposed in the completed structure. Do not weld strip seal expansion joint components to reinforcing steel or structural steel.

For staged construction, install steel retainer rail assemblies in a given subsequent stage to align with those installed in an adjacent prior stage after deflection and rotation due to deck casting of adjoining spans has occurred.

Protect metalized steel retainer rail assemblies during screeding operations per the manufacturer's recommendations. Provide temporary blocking material in the steel retainer rail seal cavities to prevent concrete intrusion during deck pour and finishing.

Loosen any temporary or sacrificial support brackets, bolts, clamps, etc. that span across the joint after initial set of concrete, but not more than two hours after conclusion of concrete placement.

Install the neoprene gland after completion of deck casting. Use a single continuous neoprene gland for the entire length of the joint. Field splicing of the neoprene gland is not permitted. Remove all joint form material and blocking material prior to installing the gland. Field install the gland in accordance with manufacturer's recommendations. Thoroughly coat all contact surfaces between the gland and the steel retainer rail seal cavities with an adhesive lubricant before setting the gland in place. Use lubricant adhesive that conforms to ASTM D4070 and is compatible with manufacture's strip seal expansion joint to attach neoprene gland to the steel retainer rails.

6.0 INSPECTION

The Engineer inspects the joint system for proper alignment and proper stud placement and attachment. If any aspect of the strip seal expansion joint is deemed unacceptable, make the necessary corrections.

When concrete is cast, use a non-aluminum, 10 foot, true-to-line straight-edge to check and grade the top of the slab on each side of the joint to ensure smooth transition between spans.

Watertight Integrity Test

- Upon completion of each strip seal expansion joint, perform a watertight integrity test on the top surface to detect any leakage. Cover the roadway section of the joint from curb to curb, or barrier rail to barrier rail, with water, either ponded or flowing, not less than 1 inch above the roadway surface at all points. Block sidewalk sections and secure an unnozzled water hose delivering approximately 1 gallon of water per minute to the inside face of the bridge railing, trained in a downward position about 6 inches above the sidewalk, such that there is continuous flow of water across the sidewalk and down the curb face of the joint.

- Maintain the ponding or flowing of water on the roadway and continuous flow across sidewalks and curbs for a period of 5 hours. At the conclusion of the test, the underside of the joint is closely examined for leakage. The strip seal expansion joint is considered watertight if no obvious wetness is visible on the Engineer's finger after touching a number of underdeck areas. Damp concrete that does not impart wetness to the finger is not considered a sign of leakage.

- If the joint system leaks, locate the place(s) of leakage and take any repair measures necessary to stop the leakage at no additional cost to the Department. Use repair measures recommended by the manufacturer and approved by the Engineer prior to beginning corrective work.

- If measures to eliminate leakage are taken, perform a subsequent watertight integrity test subject to the same conditions as the original test. Subsequent tests carry the same responsibility as the original test and are performed at no additional cost to the Department.

7.0 BASIS OF PAYMENT

Basis of payment for all strip seal expansion joints will be at the lump sum contract price for "Department Furnished Strip Seals." Strip Seal material shall be furnished by NCDOT, payment will be full compensation for furnishing any steel accessory plates for sidewalks, medians and rails, labor, tools, and incidentals necessary for installing the strip seal expansion joint in place and including all materials, labor, tools and incidentals for performing the original watertight integrity test.

CONCRETE WORK FOR JOINT REPLACEMENT (SPECIAL)

DESCRIPTION

This special provision addresses the removal, placement and finishing of concrete required for reconstructing the deck slab and, if necessary, bent diaphragms at bridge joint locations as noted in the plans.

MATERIALS

Furnish Department approved repair concrete that meets the requirements of the Concrete for Deck Repair special provision.

REMOVAL AND PREPARATION

Remove existing deck slab concrete to the limits shown in the plans. Existing concrete that is deteriorated, cracked or spalled shall be removed to sound material. Do not cut or remove the existing reinforcing steel unless otherwise noted in the plans.

Prior to concrete removal, introduce a shallow saw cut, ½" in depth, around the repair area at right angles to the concrete surface. Remove all concrete within the limits called out in the plans. If concrete removal exposes reinforcing steel beyond the limits shown in the plans, remove concrete 1" below the reinforcing steel with a 17 lb (maximum) pneumatic hammer, with points that do not exceed the width of the shank, or with hand picks or chisels, as directed by the Engineer.

Abrasive blast all exposed concrete surfaces and existing non-epoxy coated reinforcing steel in repair areas to remove all debris, loose concrete, loose mortar, rust, scale, etc. After blasting, examine the reinforcing steel to ensure at least 90% of the original diameter remains. If there is more than 10% reduction in the rebar diameter, splice in and securely tie supplemental reinforcing bars as directed by the Engineer. This might require additional removal of concrete, in order to achieve an appropriate splice length of the reinforcing steel. Touch-up epoxy coatings of exposed epoxy reinforcing steel, as directed by the Engineer.

Follow all abrasive blasting with vacuum cleaning.

Prior to placing concrete at joint repair locations, install a rigid bulkhead at the required grade and profile.

For joint repairs utilizing elastomeric concrete, attach a 5 ½" x 2 ¼" minimum block out to the rigid bulkhead. The block out shall have 1" diameter air bleed holes spaced at 3' centers along the block out to relieve air pockets and facilitate concrete consolidation. Once the concrete has cured properly, remove the block out and install elastomeric concrete as described in the Elastomeric Concrete special provision.

For joint repairs utilizing strip seals, secure the strip seal retainer rails in final position to match existing grade and cross slope. Furnish falsework to support retainer rails during installation as described in the Strip Seal special provision.

Secure screed rail guides in position to ensure finishing the surface to the required profile and cross slope. Do not treat screed rails with parting compound to facilitate their removal.

Completely clean all surfaces of grease, oil, curing compounds, acids, dirt, or loose debris within 24 hours of placing concrete. Thoroughly soak and cover existing concrete surfaces for a minimum of two (2) hours prior to placing concrete. Remove any standing water from the repair area surface prior to placing concrete

PLACING AND FINISHING

Construction joints other than those shown on the plans will not be permitted unless approved by the Engineer.

Prepare, mix, place, and finish repair concrete in accordance with this special provision and the Concrete for Deck Repair special provision.

Place the concrete monolithically in one operation. Concrete shall not be placed in layers. Sections to be reconstructed are to be filled full depth and shall progress horizontally. Deviation from this procedure shall be cause for rejection.

Stop all placement operations during periods of precipitation. Keep an adequate quantity of protective coverings at the worksite and take adequate precautions to protect the freshly placed concrete from precipitation.

When a tight, uniform surface is achieved and before the concrete becomes non-plastic, finish the top surface of the deck repair by burlap dragging or other approved method that produces an acceptable uniform surface texture.

Texture the top surface of the repair concrete by one of the following methods:

- A steel-tined tool with 1/8 inch-wide tines that will mark the finished concrete surface to a depth of 1/8 to 3/16 inch. Randomly space the markings from 3/4 to 1 1/2 inch as approved.

- A finned float having a single row of fins that will groove the finish approximately 3/16 inchwide by 1/8 inch deep. Randomly space the markings from 3/4 to 1 1/2 inch as approved. Perform this operation so the texture will be achieved while minimizing displacement of the larger aggregate particles.

- Orient the texturing perpendicular to the roadway centerline and full width of the roadway width except leave smooth strips 18 inches wide along each curb faces. Do not overlap texturing.

Texturing shall be considered incidental to the placement and finishing of the concrete. Correct all non-specification surface texturing, at no additional cost to the Department. As soon as the surface supports burlap without deformations, cover the surface with two layers of clean, wet burlap. Drain excess water from the burlap before placement. Other wet cure methods are permitted but must be approved by the Engineer prior to start of placement.

Wet cure the concrete a minimum of three (3) hours or until 4,500 psi compressive strength is obtained.

After the concrete has hardened sufficiently, test the finished surface with a straightedge that is designed, constructed, and adjusted such that it will accurately indicate or mark all floor areas which deviate from a plane surface by more than ⅛” in 10 feet. Remove all high areas in excess of ⅛” in 10 feet with an approved grinding or cutting machine. Where variations are such that the corrections extend below the limits of the top layer of grout, seal the repaired surface with an approved sealing agent. Methods for correcting low areas shall be approved by the Engineer.

LIMITATIONS OF OPERATIONS

Submit volumetric mixer size and weight data to the Engineer for review. The volumetric mixer may be allowed on the bridge deck with approval from the Engineer.

No vehicular or construction traffic is permitted on finished concrete prior to achieving a compressive strength of 4,500 psi.

If working at night, provide approved lighting

Measurement and Payment

Concrete Work for Joint Replacement will be measured and paid for at the contract unit price bid per square feet and will be full compensation for removal, containment and disposal off-site of unsound concrete, placement and finishing of repair concrete, and shall include the cost of labor, tools, equipment (excluding the volumetric mixer) and incidentals necessary to complete the repair work.

Pay Item	**Pay Unit**
Concrete Work for Joint Replacement	Square Feet

CONCRETE FOR DECK REPAIR (SPECIAL)

Description

This special provision addresses the material requirements of high early strength structural concrete to be used for reconstruction of deck slab and, if necessary, bent diaphragms as noted in the plans.

Materials

Furnish Department approved pre-packaged or bulk concrete materials in a pre-proportioned, mix (additional materials may be provided or designated as appropriate) to satisfy provisions for Class AA Concrete detailed in Article 1000-4 of the *Standard Specifications* or as otherwise noted in this special provision. Concrete mix shall meet the following requirements:

Physical Property	**Threshold Limitation**	**Test Method**
Compressive Strength (at 3 hrs.)	4,500 psi (min.)	ASTM C39/C109
Slump	4 in. (min.) 7 in. (max.)	AASHTO T119
Water to Cement Ratio	0.450 (max.)	N/A
Modulus of Elasticity	5,200 ksi (max.)	ASTM C469

(at 28 days)		
Coefficient of Thermal Expansion (at 28 days)	4.5 in./in./°F (min.) 5.5 in./in./°F (max.)	AASHTO T336
Concrete Setting Times Initial Final	 30 min. (max.) 40 min. (max)	ASTM C191

Concrete shall be capable of placement on existing concrete substrate surfaces within the following temperature limitations:

40°F (min.)

100°F (max.)

Measurement for determination of concrete material compositions shall be in accordance with Article 1000-8 of the *Standard Specifications*.

Submit pre-packaged concrete mix contents or concrete mix design, including laboratory compressive strength data, for a minimum of six (6) 4-inch diameter by 8-inch cylinders at an age of three (3) hours and six (6) 4-in diameter by 8-inch cylinders at an age of 1 day to the Engineer for review. Include test results for the slump and air content of the laboratory mix. Perform tests in accordance with AASHTO T119 and T152.

Provide aggregates that are free from ice, frost and frozen particles when introduced into the mixer.

For equipment, proportioning, and mixing of concrete compositions, see Article 1000-12 of the *Standard Specifications*, the repair concrete manufacturer's requirements, and the Volumetric Mixer special provision. Prior to beginning any work, obtain approval for all equipment to be used for joint area preparation, mixing, placing, finishing, and curing the deck repair concrete.

Measurement and Payment

No separate payment will be made for Concrete for Deck Repair. The furnishing and placing of Concrete for Deck Repair shall be incidental to applicable pay items.

VOLUMETRIC MIXER (12-18-12)

DESCRIPTION

This provision addresses the requirements for batching deck repair concrete at the point of delivery using a Mobile High-Performance Volume Mixer (MHPVM). Work shall be in

accordance with the general requirements of Section 1000-12 of the *Standard Specifications* and as amended by these provisions.

MATERIALS

Produce high early strength concrete with MHPVM equipment. Furnish project site storage facilities that will provide protection of materials in accordance with the *Standard Specifications* and all material suppliers' recommendations.

EQUIPMENT

MHPVM devices shall have prominently displayed stamped metal plate(s) from the Volumetric Mixers Manufacturers Bureau stating that the equipment conforms to the requirements of ASTM C685.

Hydraulic cement concrete shall be mixed at the point of delivery by a combination of materials and mixer unit conforming to the following:

1.) The unit shall be equipped with calibrated proportioning devices for each ingredient added to the concrete mix. The unit shall be equipped with a working recording meter that is visible at all times and furnishes a ticket printout with the calibrated measurement of the mix being produced. If at any time the mixer fails to discharge a uniform mix, production of concrete shall be suspended until such time that problems are corrected.

2.) Each unit shall have prominently displayed stamped metal plate(s) attached by the manufacturer on which the following are plainly marked: the gross volume of the transportation unit in terms of mixed concrete, the discharge speed and the mass calibrated constant of the machine in terms of volume.

3.) MHPVMs shall be calibrated by a Department approved testing agency in accordance with the manufacturer's recommendations at an interval of every 6 months or a maximum production of 2500 cubic yards, whichever comes first prior to use on the project. The yield shall be maintained within a tolerance of +/- 1% and verified using a minimum 2 cubic feet container every 500 cubic yards or a minimum of once per week.

4.) The three cubic feet initially discharged from the truck shall be discarded and not used for concrete placement. Acceptance of the concrete shall comply with the Standard Specifications except that the sample secured for acceptance testing will be taken after four cubic feet is discharged from the delivery vehicle. During discharge, the consistency as determined by ASTM C143 on representative samples taken from the mixer discharge at random intervals shall not vary by more than 1 inch. Acceptance tests shall be performed on each load. If test data demonstrates that acceptable consistency of concrete properties is being achieved, the Engineer may reduce testing requirements.

5.) MHPVM equipment shall be operated by a person who is a certified operator by the equipment manufacturer. Any equipment adjustments made during the on-site production of concrete shall be done under the direct on-site supervision of the producer's NCDOT Certified Concrete Batch Technician.

UNIFORMITY AND ACCEPTANCE

The contractor is responsible for providing a Certified Concrete Plant Technician during batching operations, and a Certified Concrete Field Technician during placing operations

MEASUREMENT AND PAYMENT

Volumetric Mixer will be paid for as lump sum and will be full compensation for furnishing the certified MHPVM devices and calibration of the equipment.

Pay Item	Pay Unit
Volumetric Mixer	Lump Sum

County : Halifax, Northampton

Line #	Item Number	Sec #	Description	Quantity	Unit Cost	Amount
			ROADWAY ITEMS			
0001	0000100000-N	800	MOBILIZATION	Lump Sum	L.S.	
0002	4400000000-E	1110	WORK ZONE SIGNS (STATIONARY)	131 SF		
0003	4420000000-N	1120	PORTABLE CHANGEABLE MESSAGE SIGN	4 EA		
0004	4423000000-N	SP	WORK ZONE DIGITAL SPEED LIMIT SIGNS	4 EA		
0005	4424000000-N	SP	WORK ZONE PRESENCE LIGHTING	24 EA		
0006	4434000000-N	SP	SEQUENTIAL FLASHING WARNING LIGHTS	28 EA		
0007	4510000000-N	1190	LAW ENFORCEMENT	2,000 HR		
0008	4600000000-N	SP	GENERIC TRAFFIC CONTROL ITEM SINGLE LANE CLOSURE	18 EA		
0009	4815000000-E	1205	PAINT PAVEMENT MARKING LINES (6")	9,120 LF		
0010	4847030000-E	1205	POLYUREA PAVEMENT MARKING LINES (6", 20 MILS)	9,120 LF		
0011	6117500000-N	SP	CONCRETE WASHOUT STRUCTURE	4 EA		
0013	8860000000-N	SP	GENERIC STRUCTURE ITEM DEPARTMENT FURNISHED EPOXY COATED REINFORCING STEEL	Lump Sum	L.S.	
0014	8860000000-N	SP	GENERIC STRUCTURE ITEM DEPARTMENT FURNISHED STRIP SEALS	Lump Sum	L.S.	
0015	8860000000-N	SP	GENERIC STRUCTURE ITEM VOLUMETRIC MIXER	Lump Sum	L.S.	
0016	8892000000-E	SP	GENERIC STRUCTURE ITEM CONCRETE WORK FOR JOINT REPLACEMENT	2,400 SF		